# **Report of Geotechnical Engineering Exploration**

# WATER MAIN NEWBERRY, INDIANA 24-0293-01G

Prepared For:
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July 11, 2024





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Mr. Nicholas Murphy American Structurepoint, Inc. 9025 River Road, Suite 200 Indianapolis, Indiana 46240

Re: Report of Geotechnical Engineering Exploration

Water Main

County Road 475 South and County Road 100 West

Newberry, Indiana

Patriot Project No. 24-0293-01G

## Dear Nicholas:

Attached is the report of our subsurface exploration for the above referenced project. This exploration was completed in general accordance with our Change Order No. 1 dated May 9, 2024, to this project.

We appreciate the opportunity to perform this geotechnical engineering exploration and are looking forward to working with you during the construction phase of the project. If you have questions regarding this report or if we may be of additional assistance regarding any geotechnical aspect of the project, please do not hesitate to contact our office.

Respectfully submitted.

Patriot Engineering and Environmental, Inc.

Akshat Saxena, PE Project Engineer William D. Dubois, PE Senior Principal Engineer

William D. Dulia

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## REPORT OF GEOTECHNICAL ENGINEERING EXPLORATION

## WATER MAIN NEWBERRY, INDIANA 24-0293-01G

## 1.0 INTRODUCTION

## 1.1 General

American Structurepoint, Inc. is planning the installation of a new water main/line/alignment to be located along County Road 475 South and County Road 100 West in Newberry, Indiana. The results of our geotechnical engineering exploration for the water main are presented in this report.

The results for our geotechnical engineering exploration for the Westgate Wastewater Treatment Plant (WWTP) will be conveyed in a follow-up report with the same project number.

## 1.2 Purpose and Scope

The purpose of this investigation is to determine the general near surface and subsurface conditions along the referenced alignment and to provide recommendations to aid in the design and construction of the planned water line. This was achieved by drilling twenty-five (25) soil borings and by conducting laboratory tests on samples taken from the borings. Originally, twenty-seven (27) borings were planned for the water main; however, two (2) borings in the vicinity of White River were not drilled due to vegetation.

#### 2.0 PROJECT INFORMATION

The proposed project includes replacing the existing water main alignment along County Road 475 South and County Road 100 West near Newberry, Indiana, with a new 14-inch ductile iron section. We understand that the invert of the new water line will be about 6 feet below existing grades and will be constructed using open cut excavations and jack and bore methods.

The above narrative is based upon information provided to *Patriot*. This represents the most current information available at the time of issuance of this report. Our assumptions and the recommendations set forth in this report are therefore directly related to this information. *Patriot* should be notified immediately if the provided information provided to us changes during the design development. We cannot be responsible if changes are made to the project, and we are not allowed to determine if our recommendations remain valid.

#### 3.0 EXPLORATIONAL PROCEDURES

#### 3.1 Field Work

A total of twenty-five (25) soil borings were drilled, sampled, and tested at the project site between May 23 and May 29, 2024, at the approximate locations shown on the Soil Boring Location Map in Appendix A. All depths are given as feet below the existing ground surface. Borings B-101 and B-102 (near the White River) were not drilled due to bushes and trees.

The borings were advanced using 3¼" I.D. (inside diameter) hollow-stem augers. Samples were recovered in the undisturbed material below the bottom of the augers using the standard drive sample technique in accordance with ASTM D 1586-74. A 2" O.D. (outside diameter) by 1³/8" I.D. split-spoon sampler was driven a total of 18 inches with the number of blows of a 140-pound hammer falling 30 inches recorded for each 6 inches of penetration. The sum of blows for the final 12 inches of penetration is the Standard Penetration Test result commonly referred to as the N-value (or blow-count). Where the Split-spoon sampler is advanced less than 6 inches in 50 blows, it is indicated as: 50 / (number of inches advanced per 6-inch interval). Split-spoon samples were recovered at 2.5-feet intervals, beginning at a depth of 1 foot below the existing surface grade, extending to a depth of 10 feet, and at 5-feet intervals thereafter to the termination of the boring. Water levels were monitored at each borehole location during drilling and upon completion of the boring. The boreholes were backfilled with auger cuttings prior to demobilization.

Upon completion of the boring program, the samples retrieved during drilling were returned to *Patriot*'s soil testing laboratory where they were visually examined and classified. A laboratory-generated log of each boring was prepared based upon the driller's field log, laboratory test results, and our visual examination. Test boring logs and a description of the classification system are included in Appendix A in this report. Indicated on each log are the primary strata encountered, the depth of each stratum change, the depth of each sample, the Standard Penetration Test results, groundwater conditions, and selected laboratory test data. The laboratory logs were prepared for each boring giving the appropriate sample data and the textural description and classification.

## 3.2 Laboratory Testing

Representative samples recovered in the borings were selected for testing in the laboratory to evaluate their physical properties and engineering characteristics. Laboratory analyses included natural moisture content determinations (ASTM D 2216), and an estimate of the cohesive soil strength was determined by utilizing a hand penetrometer (qp). The results of all laboratory tests are summarized in Section 4.2 and are shown on the boring logs and laboratory data sheets as appropriate.

#### 4.0 SITE AND SUBSURFACE CONDITIONS

## **4.1 Alignment Conditions**

Patriot visited the referenced alignment site on May 16, 2024, to mark the boring locations and to make visual observations within the limits of the proposed project footprint. The start of the water main alignment is located along County Road 475 South, immediately north of Doans Creek and end of the water main is situated near the intersection of County Road 100 West and County Road 710 South. The surrounding area is mainly agricultural development. The elevation increases from 501 feet at the start of the alignment to 615 feet at the end.

## 4.2 Subsurface Conditions

Our interpretation of the subsurface conditions is based upon widely spaced soil borings drilled at the approximate locations shown on the Boring Location Map in Appendix A. The following discussion is general; for more specific information, please refer to the boring logs presented in Appendix A. The dashed stratification lines shown on the soil boring logs indicate approximate transitions between soil types. In situ stratification changes could occur gradually or at different depths. All depths discussed below refer to depths below the existing ground surface.

<u>Crushed Stone</u> – Crushed stone was encountered at the surface in eight<sup>1</sup> (8) borings and underneath asphalt in four<sup>2</sup> (4) borings. The crushed stone thickness varied from 10 to 12 inches in the borings.

<u>Topsoil</u> – Topsoil, a surficial layer of material that is a blend of silts, sands, and clays, with varying amounts of organic matter, was observed in thirteen<sup>3</sup> (13) borings at ground surface and the thickness ranged between 10 and 14 inches.

<u>Asphalt</u> – Asphalt was noted at the surface in four<sup>2</sup> (4) borings, and the thickness was either 4 or 5 inches.

Native Cohesive Soils (CL and CL-ML) — Native cohesive soils observed within our borings were classified as soft to hard silty clays, sandy clays, and sandy silty clays. Standard Penetration Test N-values (blow counts) in this material varied from 4 to 89 blows per foot (bpf). Soft clayey soils (indicating yielding/compressible soils) were encountered in boring B-109 at a depth of about 6 feet. The native clayey materials have moisture contents ranging from 10 to 35 %.

<u>Native Granular Soils (SP-SM and SM)</u> – Native granular soils encountered within our borings were classified as loose to medium dense sands and silty sands. Standard Penetration Test N-values in this material varied from 5 to 23 bpf.

D-125 tillough D-120

<sup>&</sup>lt;sup>1</sup> B-103, B-108, B-112, B-114, B-116, B-117, B-118, B-122

<sup>&</sup>lt;sup>2</sup> B-123 through B-126

<sup>&</sup>lt;sup>3</sup> B-104 through B-107, B-109 through B-111, B-113, B-115, B-119 through B-121, B-127

<u>Highly Weathered Shale</u> - Highly weathered shale was encountered underlying the overburden soils at 8.5 feet in B-117. Standard Penetration Test N-value for the sample at 8.5 feet was 55 bpf.

<u>Sandstone</u> - Highly weathered sandstone was encountered underlying the overburden soils at 8.5 feet in B-122. Splitspoon refusal was noted for the sample at 8.5 feet.

## 4.3 Groundwater Conditions

Groundwater was observed in four (4) borings during drilling and in one (1) boring on completion above the cave-in depths when augers were removed from the boreholes. Refer to Table 1 for groundwater depths.

**Table 1. Groundwater depths** 

Boring No.	Groundwater depth during drilling	Groundwater depth at completion
Borning No.	(feet)	(feet)
B-106, B-112	8.5	Dry
B-116	10	0 (ground surface)
B-120	1	Dry

The term groundwater pertains to any water that percolates through the soil found on site. This includes any overland flow that permeates through a given depth of soil, perched water, and water that occurs below the "water table", a zone that remains saturated and water-bearing year-round.

It should be recognized that fluctuations in the groundwater level should be expected over time due to variations in rainfall and other environmental or physical factors. The true static groundwater level can only be determined through observations made in cased holes over a long period of time, the installation of which was beyond the scope of this exploration.

## **5.0 DESIGN RECOMMENDATIONS**

## 5.1 Basis

Our recommendations are based on data presented in this report, which include soil borings, laboratory testing and our experience with similar projects. Subsurface variations that may not be indicated by a dispersive exploratory boring program can exist on any site. If such variations or unexpected conditions are encountered during construction, or if the project information is incorrect or changed, we should be informed immediately since the validity of our recommendations may be affected.

## 5.2 Subsurface Utilities

As previously mentioned, we understand that the proposed 14-inch diameter water main replacement will bear approximately 6 feet below existing grades. We understand that installation will include either open cut construction or trenchless technology construction (directional drilling).

## 5.2.1 Open Cut Construction

We understand that excavations up to 6 feet below the existing grade will be performed for the proposed receiving pits and small sections of the proposed water main. The feasibility of performing open-cut excavations will be influenced by several factors including easement widths/excavation limits, groundwater conditions and control, and the location of existing utilities and structures. The contractor shall evaluate the need for temporary retention (in conjunction with our recommendations in this section) prior to excavation and is completely responsible for selection, design, installation, and satisfactory performance of the retention system. The design of the retention system should not only take into account the lateral forces but also the tolerable lateral deflections. In areas where the excavation is in close proximity to existing structures, we recommend that a preconstruction survey of these structures be performed prior to construction. In addition, periodic survey monitoring (of both the structure and retention system) during construction is recommended. The contractor's "responsible person" should also establish a minimum lateral distance from the crest of the slope or excavation for all spoil piles and construction equipment. Likewise, the contractor's "responsible person" should establish protective measures for exposed faces.

The following are our recommendations for temporary open-cut excavation slopes based on subsurface conditions revealed by the borings in this exploration. These recommendations are based on the assumption that groundwater will be effectively controlled by dewatering. Inadequate groundwater management could cause unstable slopes that may require additional flattening of slopes, installation of intermediate benches and possibly a retention system.

All open-cut excavations deeper than 5 feet (up to 20 feet) should, as a minimum, be performed per current OSHA Excavation Regulations. Open-cut excavations deeper than 20 feet should be designed by a registered professional engineer.

The location of existing utilities to remain operational during construction and adjacent structures (including basement information) should be taken into consideration in evaluating the feasibility of an open cut excavation. Where sufficient space is available, the excavation slopes should, as a minimum, be laid back in accordance with current OSHA Excavation Regulations. If open-cut excavation is not feasible, consideration could be given to use of trench boxes for temporary retention or a combination of open-cut excavation and trench boxes. In areas where temporary retention is required, it is important that the retention system be installed prior to the excavation.

Most of the borings performed along the water main alignment encountered layers of loose to medium dense sands at various depths which are expected to be free-flowing and will tend to readily cave and/or slough into excavations.

Highly weathered shale and sandstone was encountered in borings B-117 and B-122, respectively, at 8.5 feet below the existing ground surface (Elevation 539.5 to 532.5 feet, respectively). Although our drill rig was able to drill thorough most of the highly weathered shale, the contractor should select appropriate equipment to excavate the anticipated material. Additionally, highly weathered zones of sandstone can be excavated with conventional equipment. However, it should be noted that ripping in the slightly weathered and unweathered shales sandstones will be needed. Furthermore, the excavation become progressively more difficulty with increase in depth and decrease in weathering.

It is recommended that the temporary excavation slopes be examined periodically to evaluate potential destabilizing effects. The presence of perched water within the walls of the temporary excavations (wet seams and layers) could require flatter temporary slopes than those recommended. The stockpiling of excavated soils and rock at/near the top of the excavation can impact the stability of the excavation slopes. We recommend that the excavated soils and rock be stockpiled a minimum 10 feet away from the top of the excavation to minimize surcharge effects on the slope. The operation/storage of heavy construction equipment near the top of the excavation slope and its impact on the stability of the slope should be further evaluated to determine appropriate setbacks.

Excavations in the vicinity of slopes should be performed with extreme care. In these areas, we recommend the excavations be performed in maximum 20 foot long sections to minimize disturbance of the slope. Each section should be backfilled prior to opening up the adjacent one. Temporary retention (as needed) should be in-place prior to beginning the excavation. We recommend flowable fill be used to backfill excavations in the vicinity of slopes.

## 5.2.2 Trenchless Installation

As mentioned previously, it is our understanding that the proposed invert water main depth is approximately 6 feet below existing grades or shallower. *As indicated earlier,* highly weathered shale and sandstone were noted in B-117 and B-122, respectively, at 8.5 feet (Elevation 539.5 to 532.5 feet, respectively). The contractor should choose an appropriate trenchless technology based on the soil and weathered rock conditions presented in this report and the proposed invert elevations.

## 5.2.3 Pipe Bedding

In general, pipe bedding should consist of relatively clean, well-graded aggregate and meet all local requirements. The excavated soils for this project are generally not suitable for use as pipe bedding. We offer these general comments on pipe bedding for consideration.

It is recommended that granular pipe bedding material be used and consist of well-graded sand and gravel with no more than 10% passing the No. 200 sieve. This granular material should not be less than 6 inches in thickness below the bottom of the pipe and should extend to a height of at least 12 inches above the top of the pipe. This material should be moisture conditioned to within + 2% of its optimum moisture content and compacted to at least 95% of Standard Proctor maximum dry density, ASTM D 698. The compaction of material above the pipe should be performed with caution to prevent pipe damage. The remaining trench backfill above the granular zone previously described, shall consist of backfill as described in the following section.

## **6.0 CONSTRUCTION CONSIDERATIONS**

## 6.1 Structural Fill and Fill Placement Control

Structural fill, defined as any fill which will support structural loads, should be clean and free of organic material, debris, deleterious materials, and frozen soils. Samples of the proposed fill materials should be tested prior to initiating the earthwork and backfilling operations to determine the classification, the natural and optimum moisture contents and maximum dry density and overall suitability as a structural fill. **Structural fill should have a Liquid Limit (LL) less than 40 and a Plasticity Index (PI) between 10 and 20.** 

## 6.1.1 Existing Site Materials

Regarding the suitability of the on-site soils for use as structural fill, the upper few feet of these soils are expected to be wetter than the estimated optimum moisture contents, depending on the prevailing weather conditions at the time of construction. Therefore, scarification and/or drying may be required to reduce the moisture content of the soils to achieve adequate compaction of the clays and proper strength.

## 6.1.2 Fill Placement Control

Structural fill supporting, around and over utilities should be compacted to at least 95 percent (%) of its maximum Standard Proctor dry density (ASTM D-698) for utilities underlying structural areas (i.e. buildings, pavements, sidewalks, etc...). However, the minimum compaction requirement can be reduced for backfill around and over the utilities to 95 percent (%) of the maximum Standard Proctor dry density where utilities underlie greenbelt areas (i.e. grassy lawns, landscaping, etc...). It is recommended that a clean well-grade granular material be utilized as the bedding material, as well as the backfill material around and over the utility lines.

To achieve the recommended compaction of the structural fill, we suggest that the fill be placed and compacted in layers not exceeding 8 inches in loose thickness (the loose lift thickness should be reduced to 6 inches when utilizing small hand compactors) and within the range of 2 percentage (%) points below or above the optimum moisture content value. All fill placement should be monitored by a *Patriot* representative. Each lift should be tested for proper compaction at a frequency of at least one (1) test for every 50 lineal feet of utility installation.

## 6.2 Groundwater

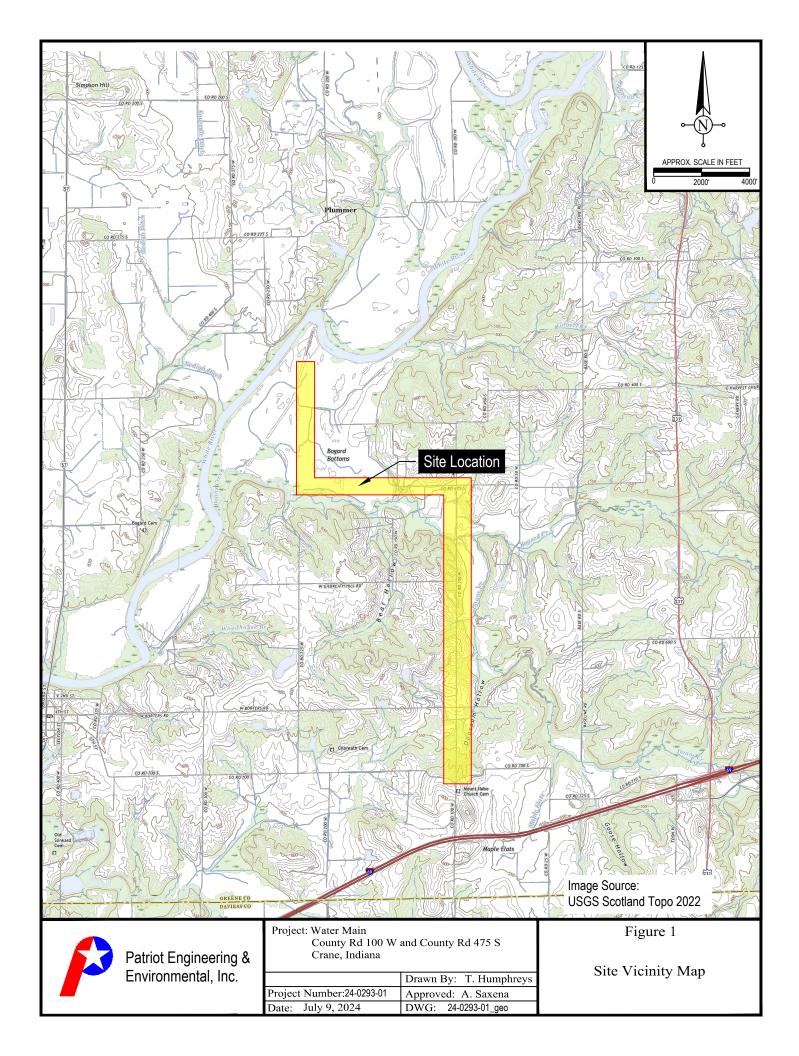
Groundwater was observed in four (4) borings during drilling between 1 and 10 feet. At completion, groundwater was noted in one (1) boring at the ground surface; these depths are within the planned excavation depths. Consequently, groundwater infiltration will occur within the excavations on this site. Additionally, the groundwater level will rise above the current levels during wet periods and flood events. Significant inflow can be expected in deeper excavations requiring more aggressive dewatering techniques, such as well or wellpoint systems. The type of dewatering method necessary for this project can only be adequately determined during construction.

## 7.0 LIMITS OF EXPLORATION

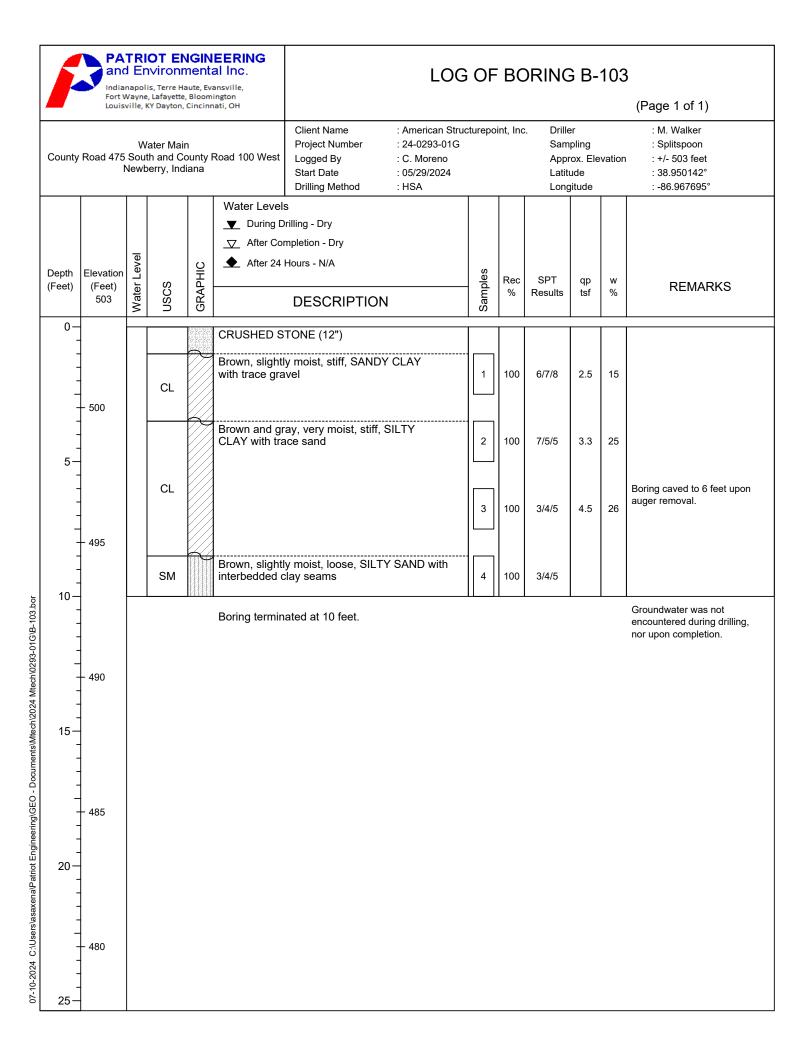
The recommendations provided herein were developed from the information obtained in the test borings, which depict subsurface conditions only at specific locations. Subsurface conditions at other locations may differ from those occurring at the specific drill sites. The nature and extent of variations between borings may not become evident until the time of construction. If variations become evident, it will be necessary to reevaluate the recommendations of this report after performing on-site observations during construction and noting the characteristics of any variation.

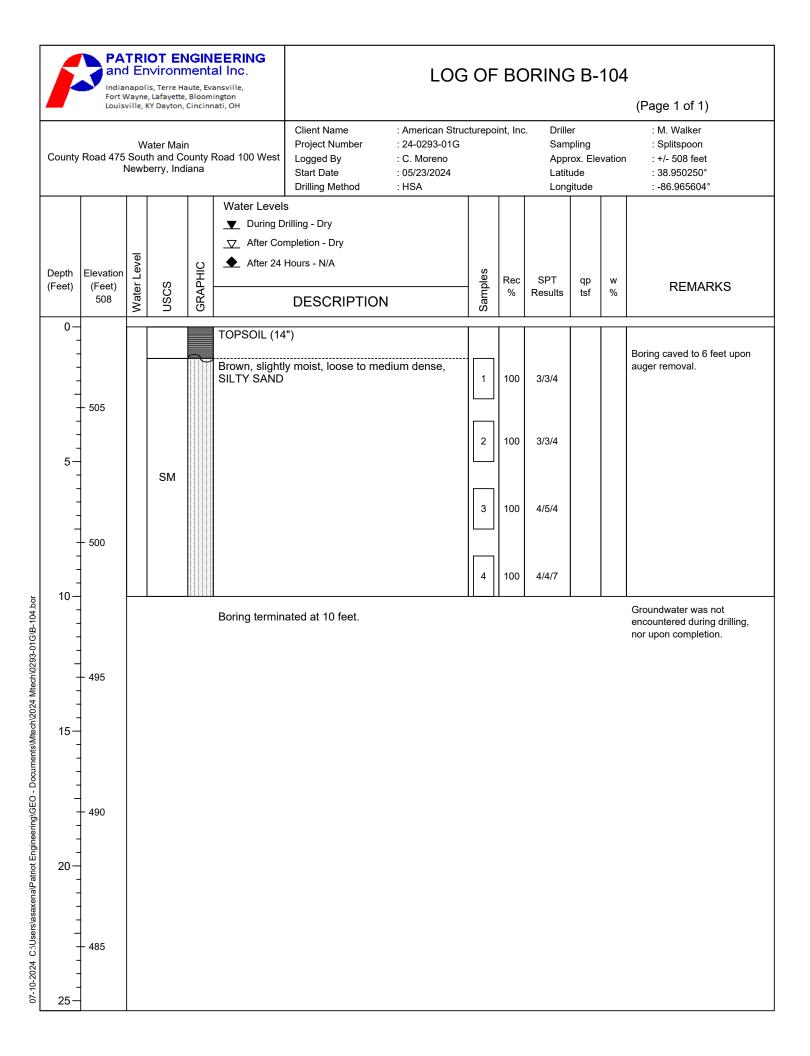
Our professional services have been performed, findings obtained, and recommendations prepared in accordance with generally accepted geotechnical engineering principles and practices. This warranty is in lieu of all other warranties either expressed or implied. This company is not responsible for the independent conclusions, opinions or recommendations made by others based on the field and laboratory data presented in this report. The scope of our services did not include any environmental assessment or investigation for the presence or absence of wetlands, hazardous or toxic materials in the soil, groundwater, or surface water within or beyond the site studied.

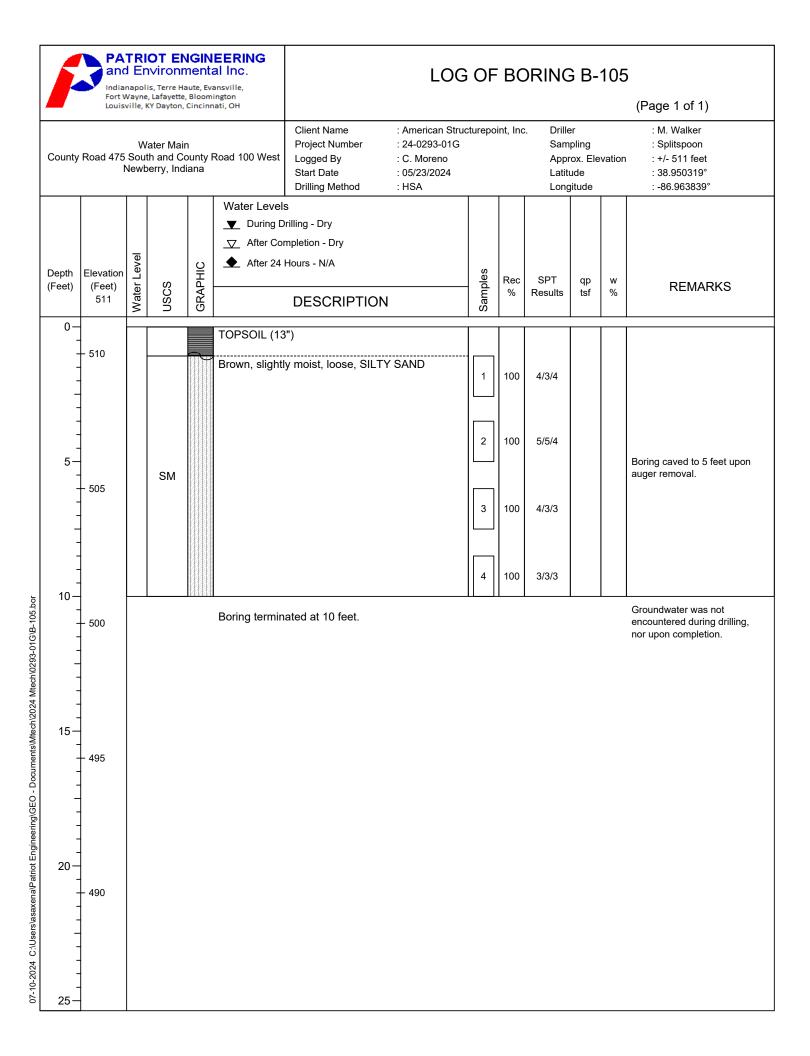
APPENDIX A	
Site Vicinity Map	
Soil Boring Location Map	
Boring Logs	
Boring Log Key	
Unified Soil Classification System (USCS)	

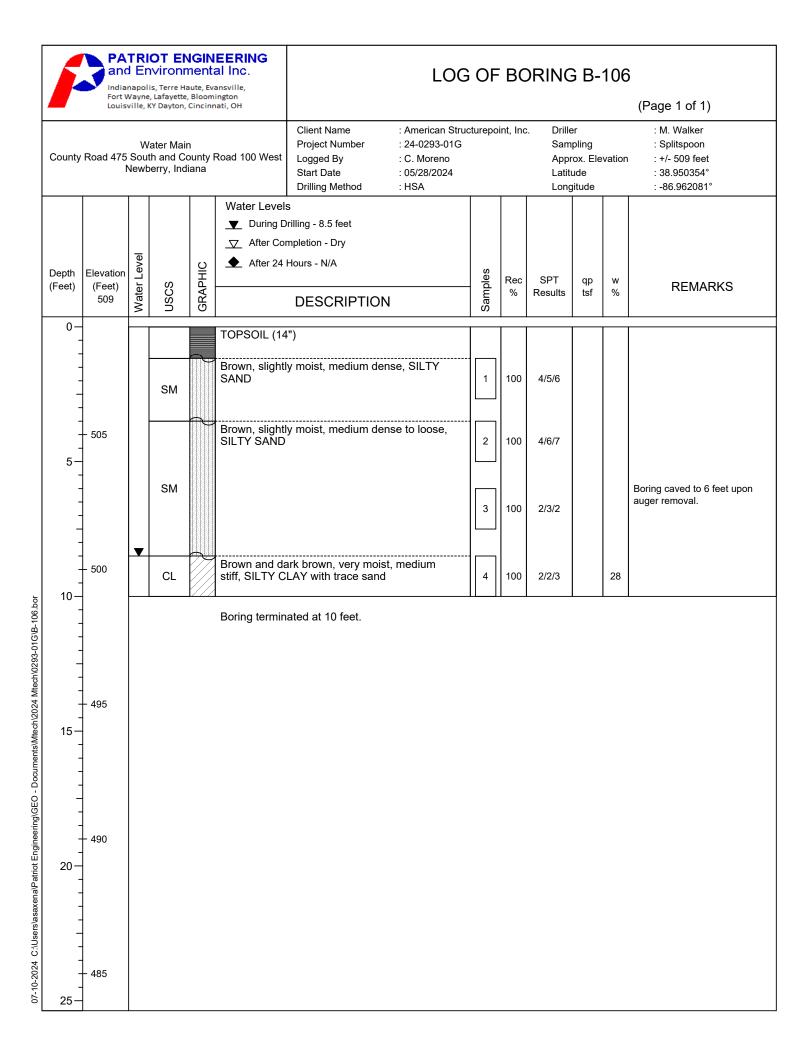


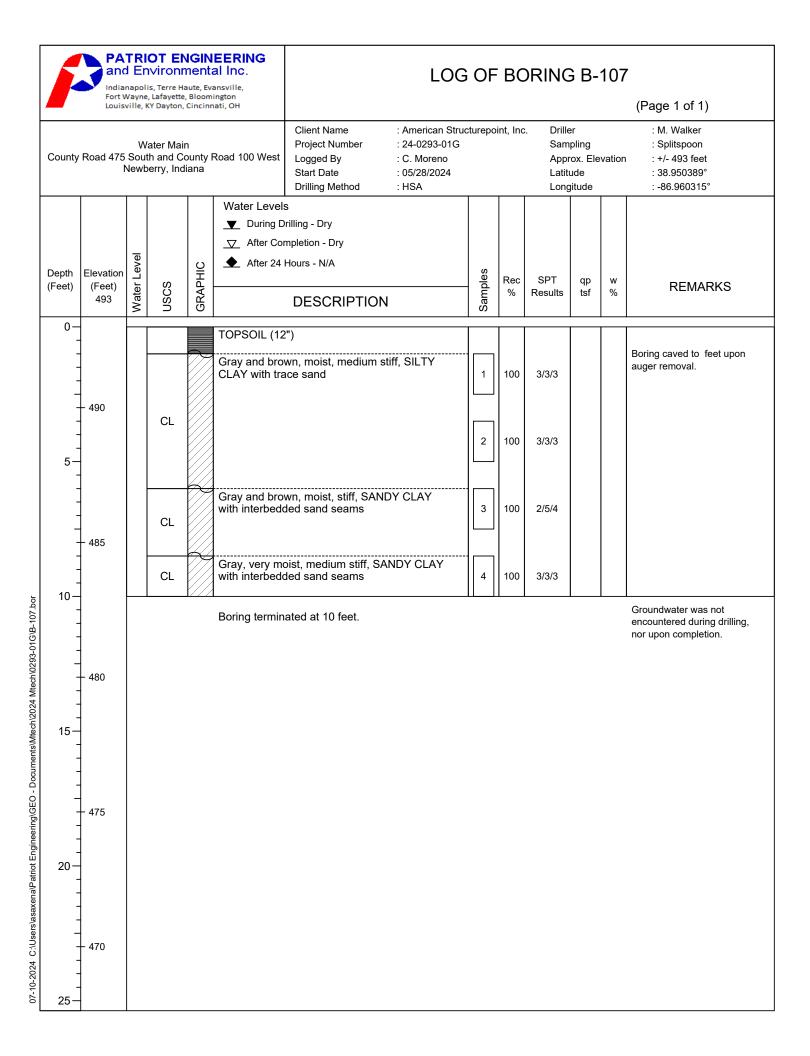


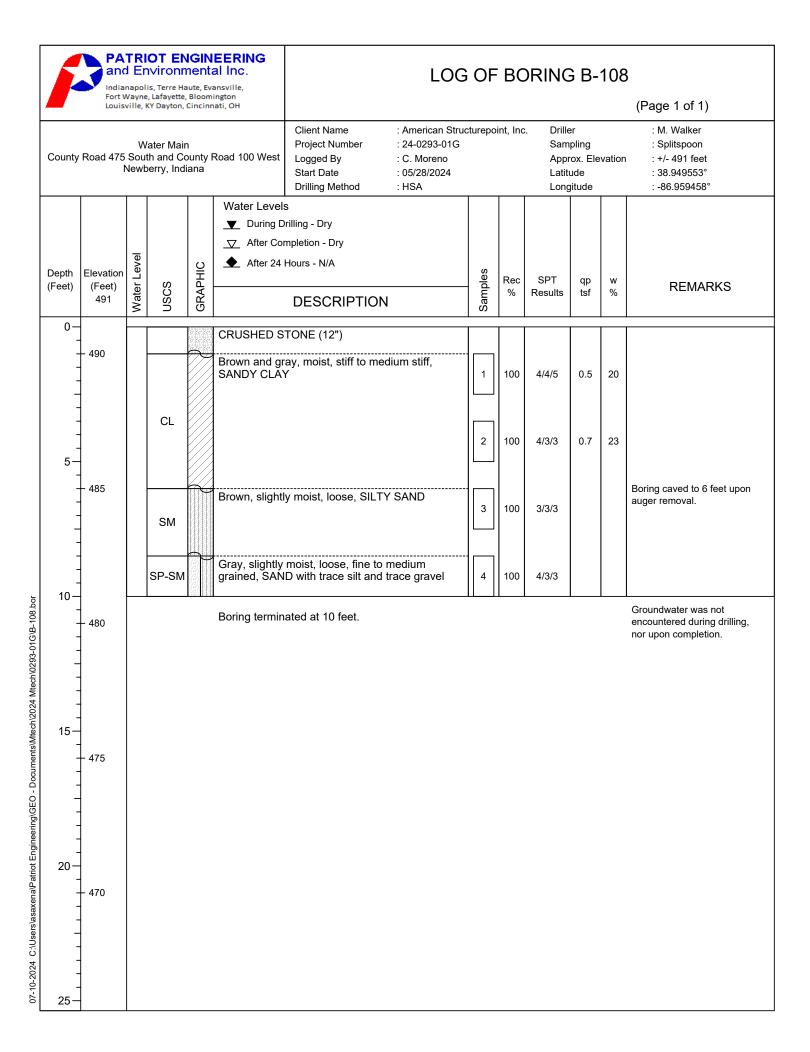




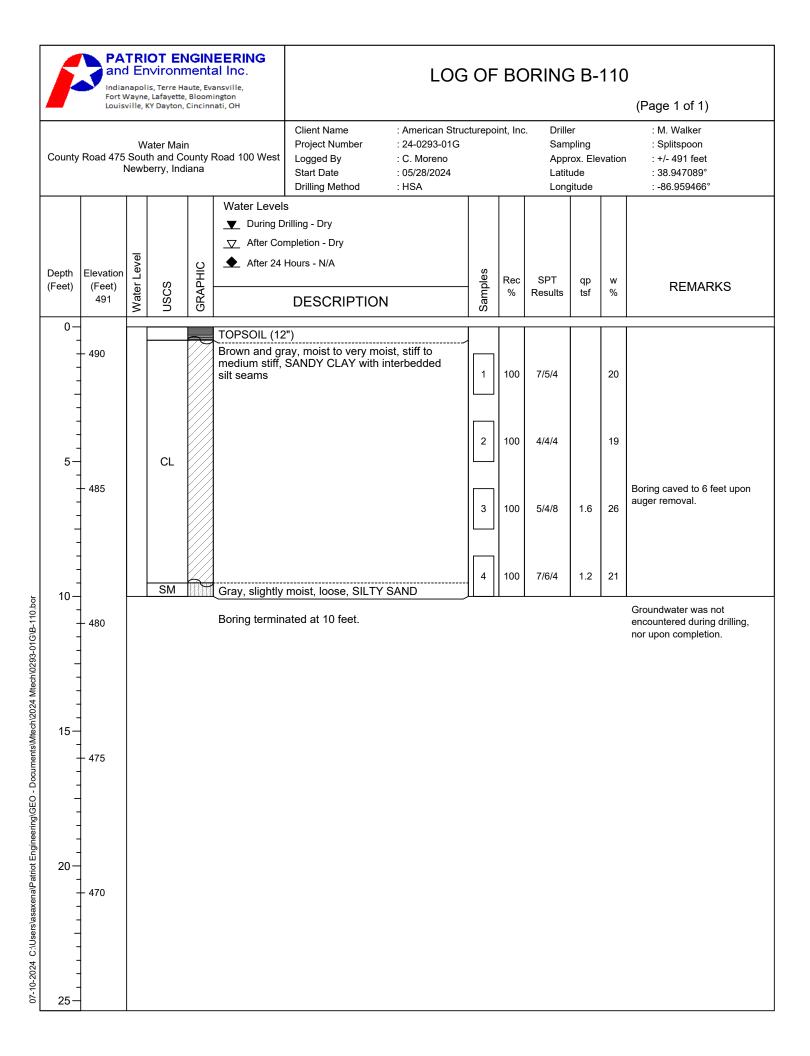


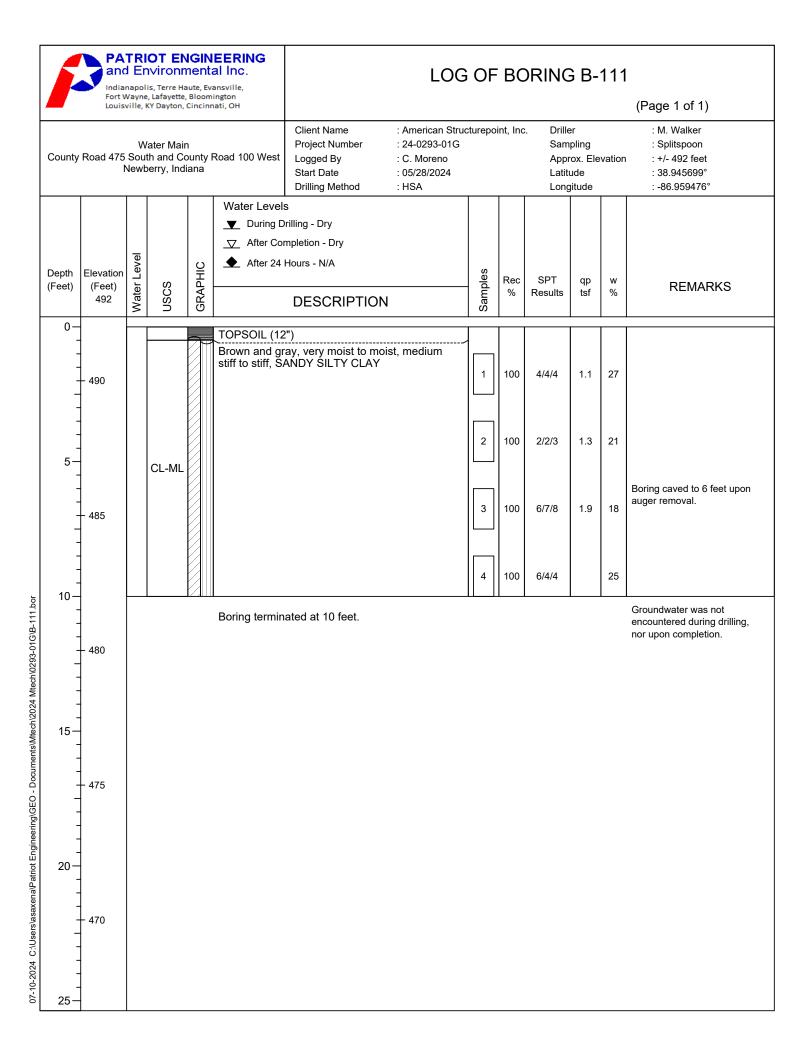


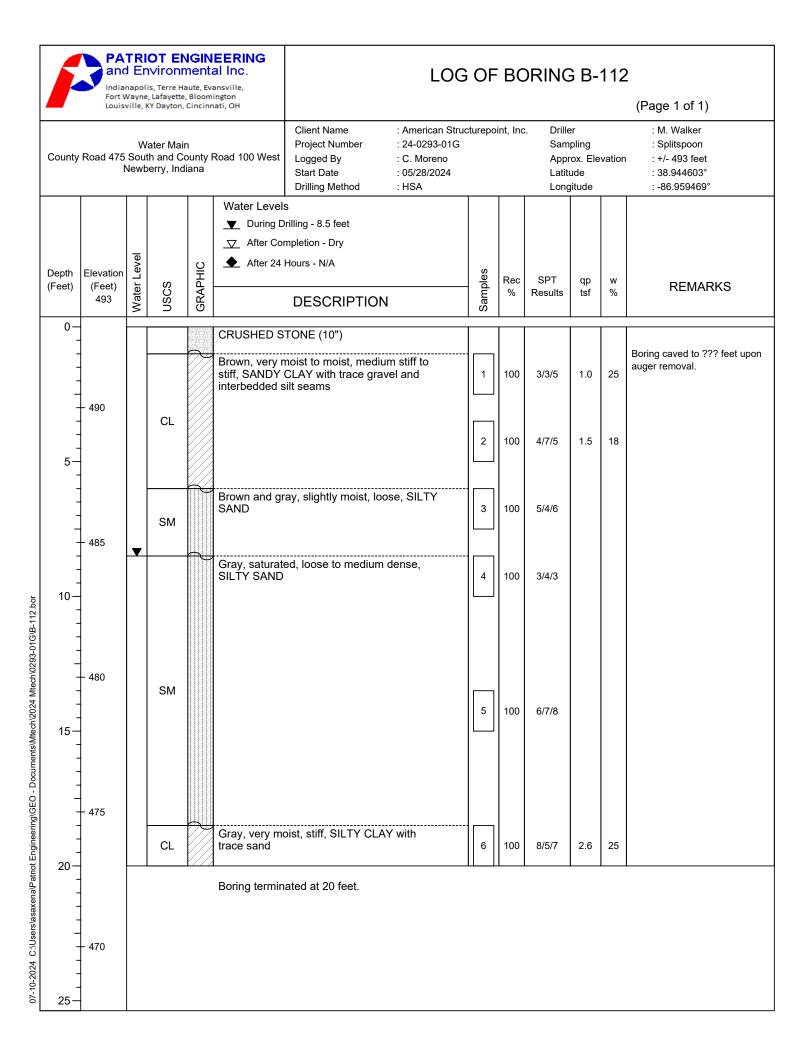


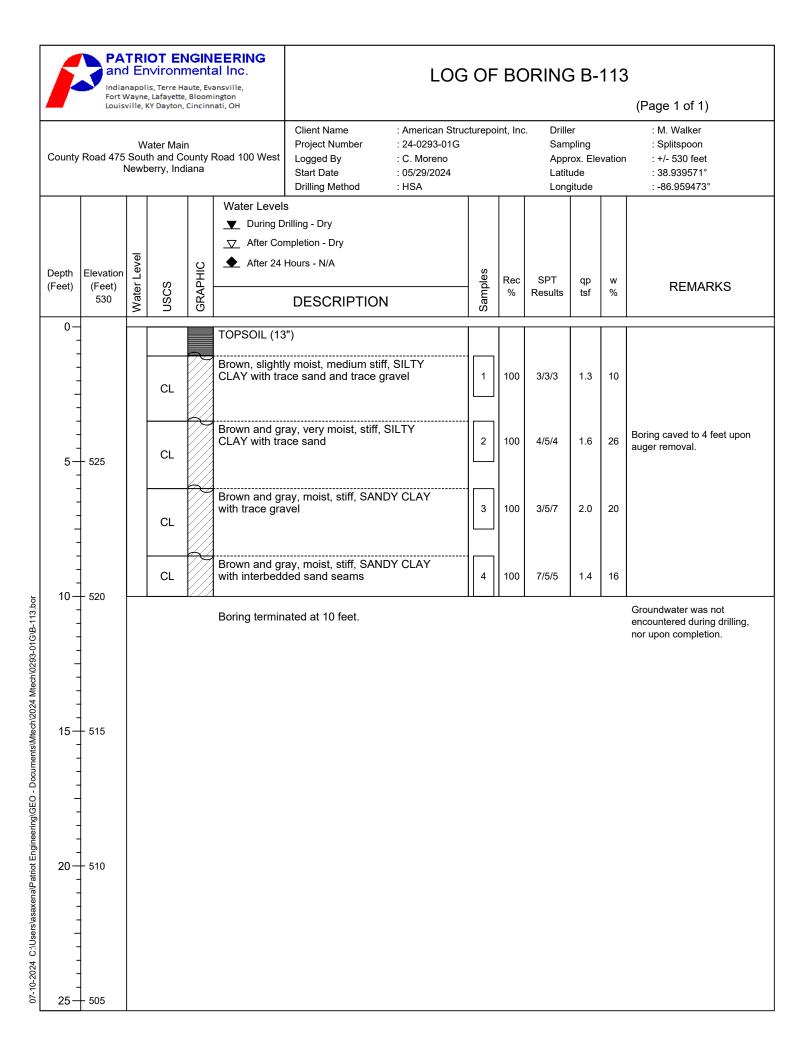


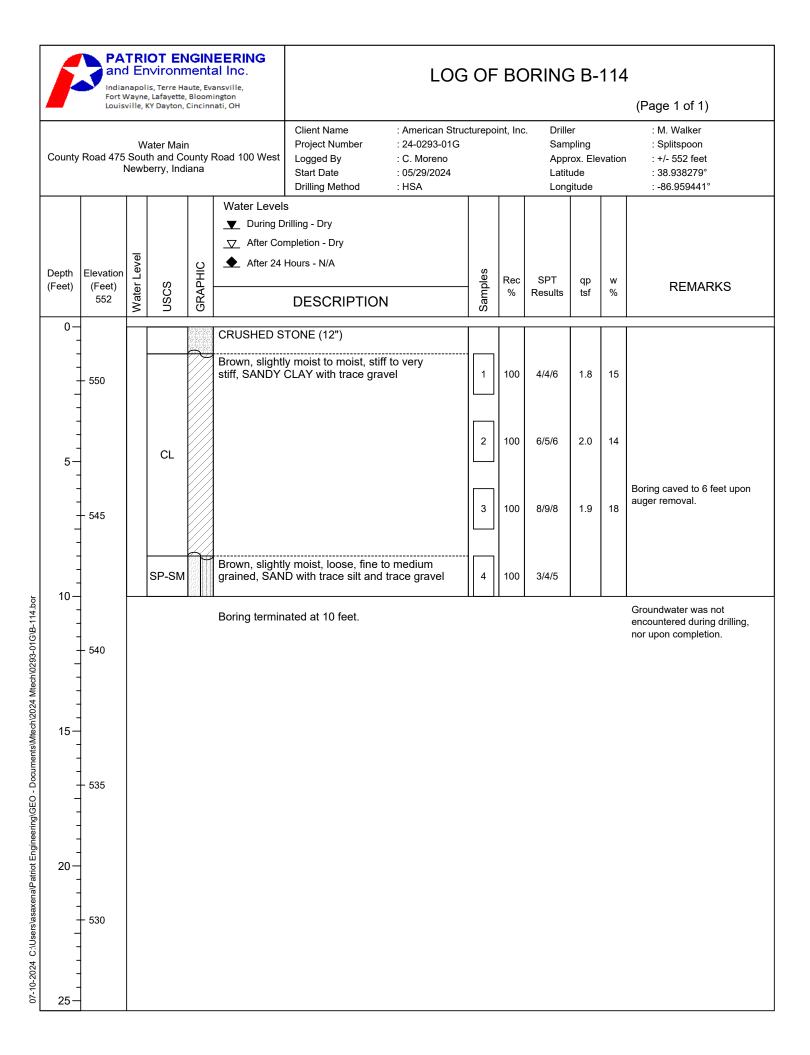
	PATRIOT ENGINEERING and Environmental Inc. Indianapolis, Terre Haute, Evansville, Fort Wayne, Lafayette, Bloomington					LOG OF BORING B-109							
Count	Water Main County Road 475 South and County Road 100 West Newberry, Indiana					Client Name : American Structurepoint, Inc. Driller Project Number : 24-0293-01G Samplir Logged By : C. Moreno Approx. Start Date : 05/28/2024 Latitude			pling ox. Ele	. Elevation : +/- 491 feet			
Depth (Feet)	1	Water Level	nscs	GRAPHIC	Water Level  ▼ During D  ▼ After Co  ◆ After 24	Samples	Rec %	SPT Results	qp tsf	w %	: -86.959480° REMARKS		
0	- - - - - - - -		CL		Brown and gr	TOPSOIL (10") Brown and gray, moist to very moist, medium stiff to stiff, SILTY CLAY with trace sand					1.2	22	
5	- - - - - + 485		OL.					2	100	3/5/5	1.0	27	
	-		CL		trace sand Gray, slightly	oist, soft, SILTY CL	se, fine to	3	100	2/2/2		35	
ch\0293-01G\B-109.bor	- - - - - - - - - - - - - -		SP-SM		medium grair trace gravel	ned, SAND with trac	e silt and	4	100	11/8/11			
EO - Documents/Mtech/2024 Mt.	- - - - - - - - - - - - - - - - - - -		CL		Brown and gr with trace sar	ray, moist, stiff, SILT	Y CLAY	5	100	5/6/7	4.2	22	Boring caved to 14 feet upon auger removal.
iot Engineering\G			CL		Gray and bro with trace sar	own, moist, stiff, SIL <sup>-</sup> nd	TY CLAY	6	100	4/7/8	1.1	22	
-10-2024 C∴Users∖asaxena∖Patr	- - 470 - - - - -				Boring termin	nated at 20 feet.							Groundwater was not encountered during drilling, nor upon completion.
≥ 25													

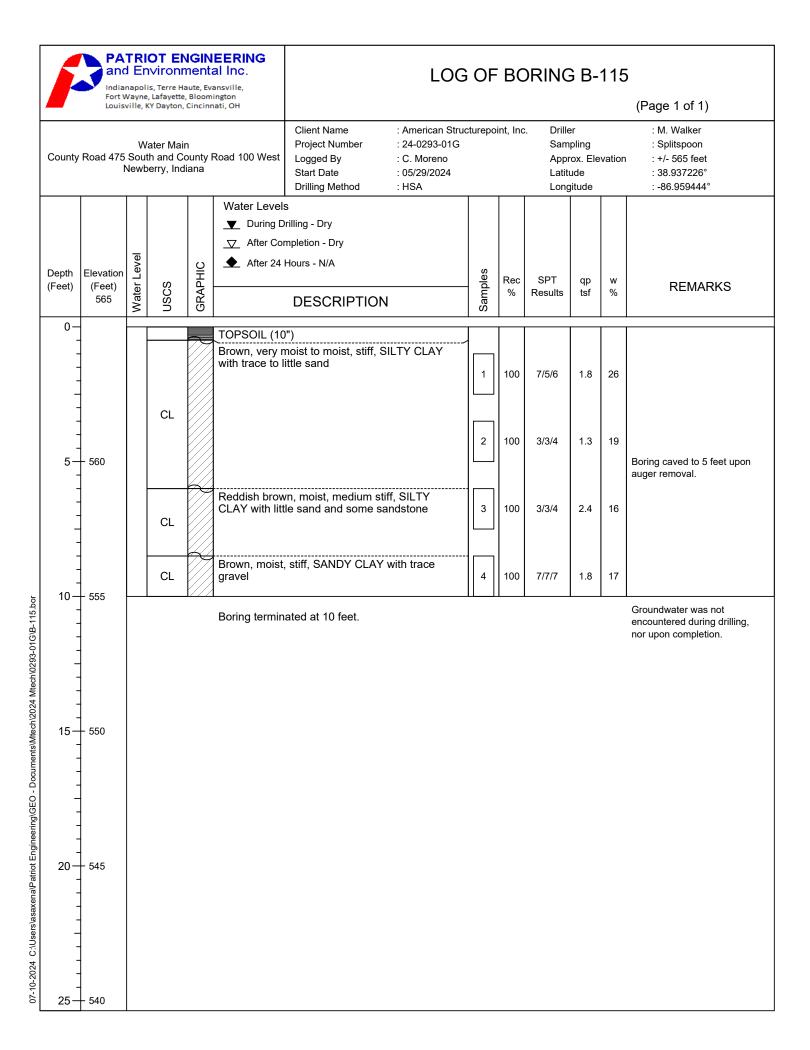


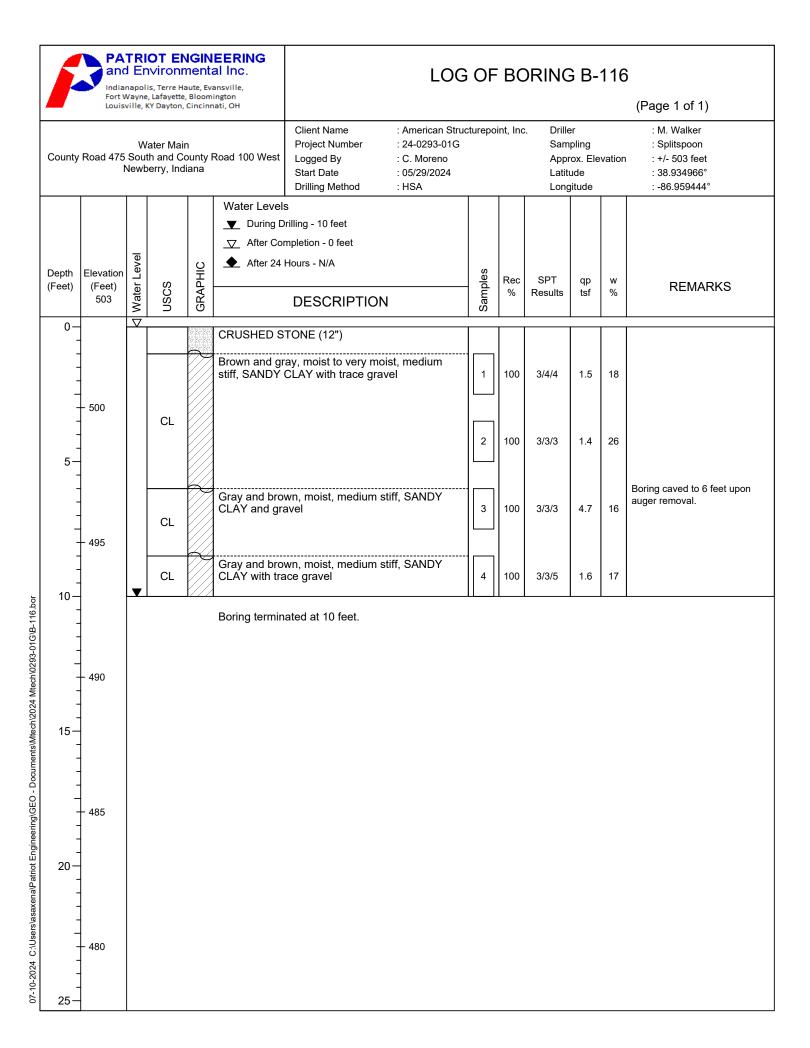


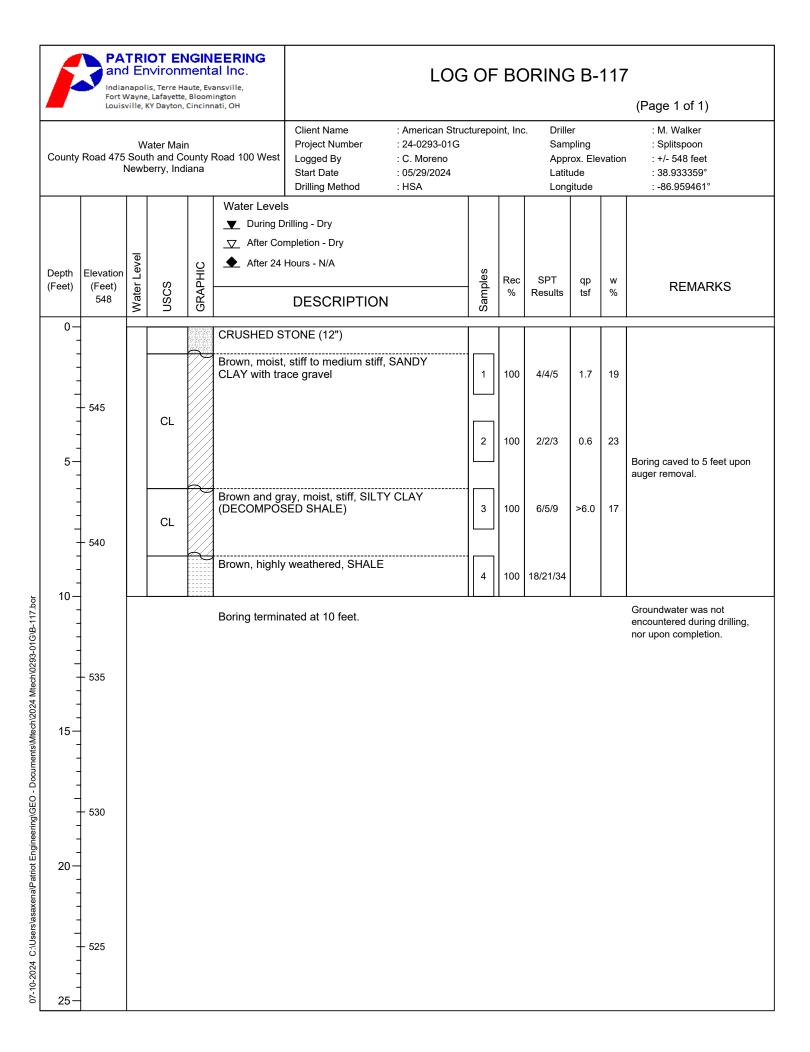


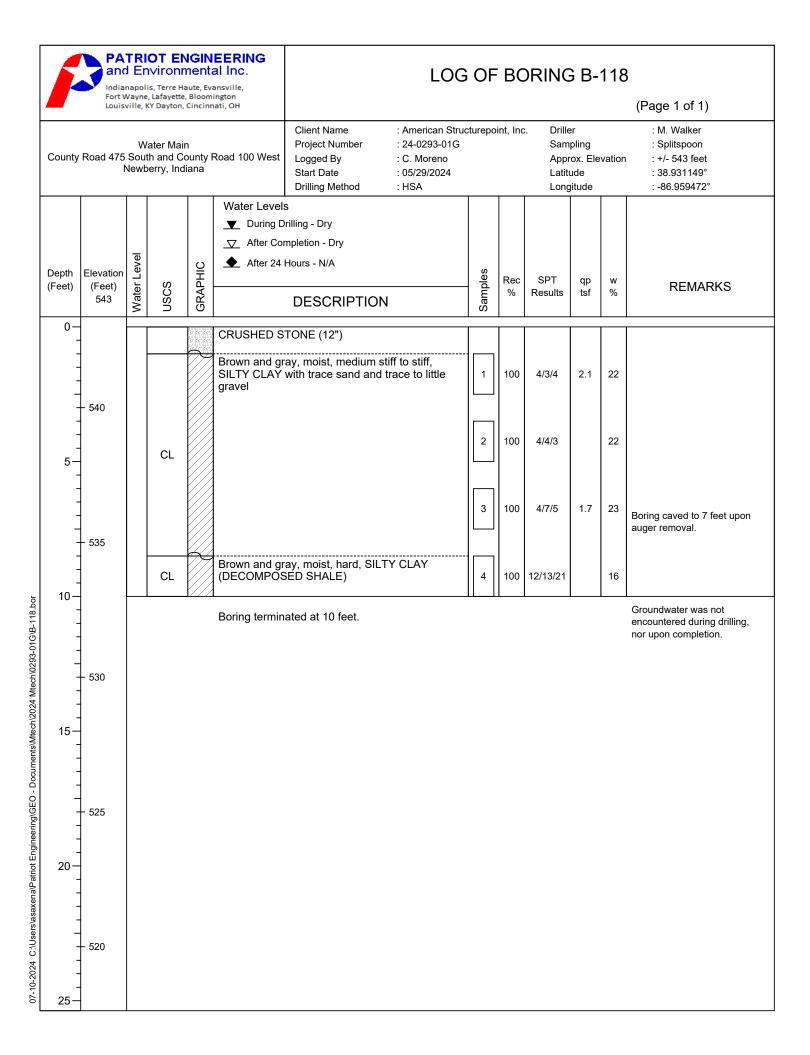


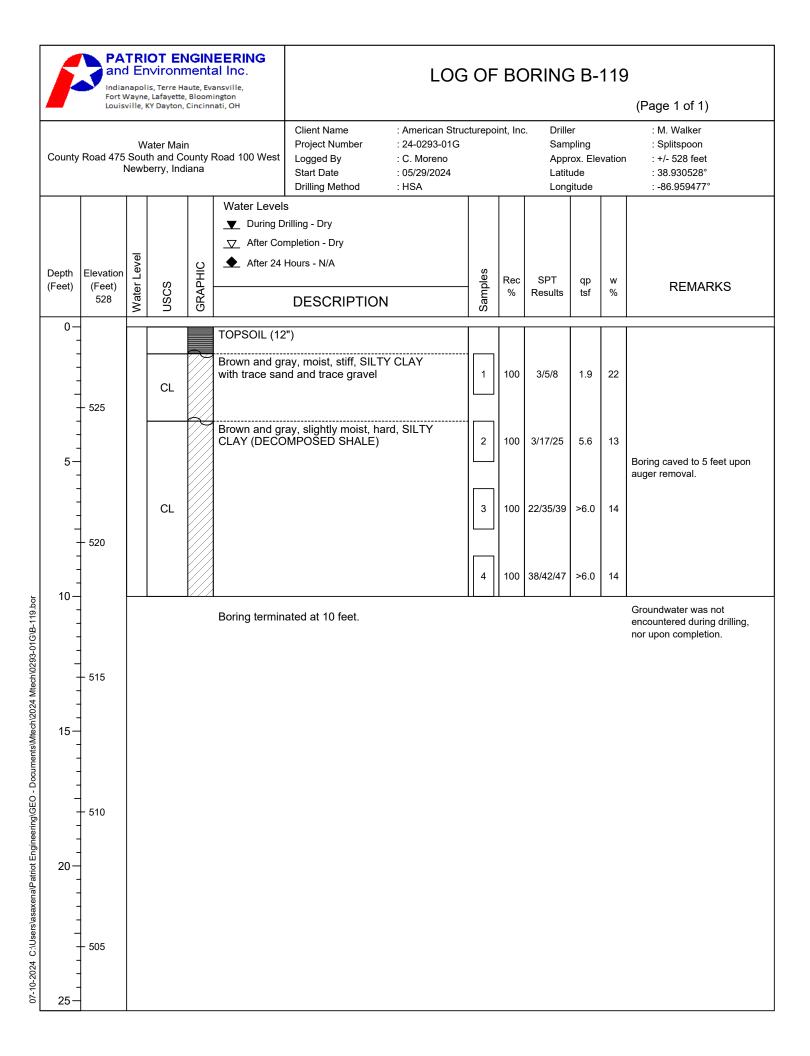


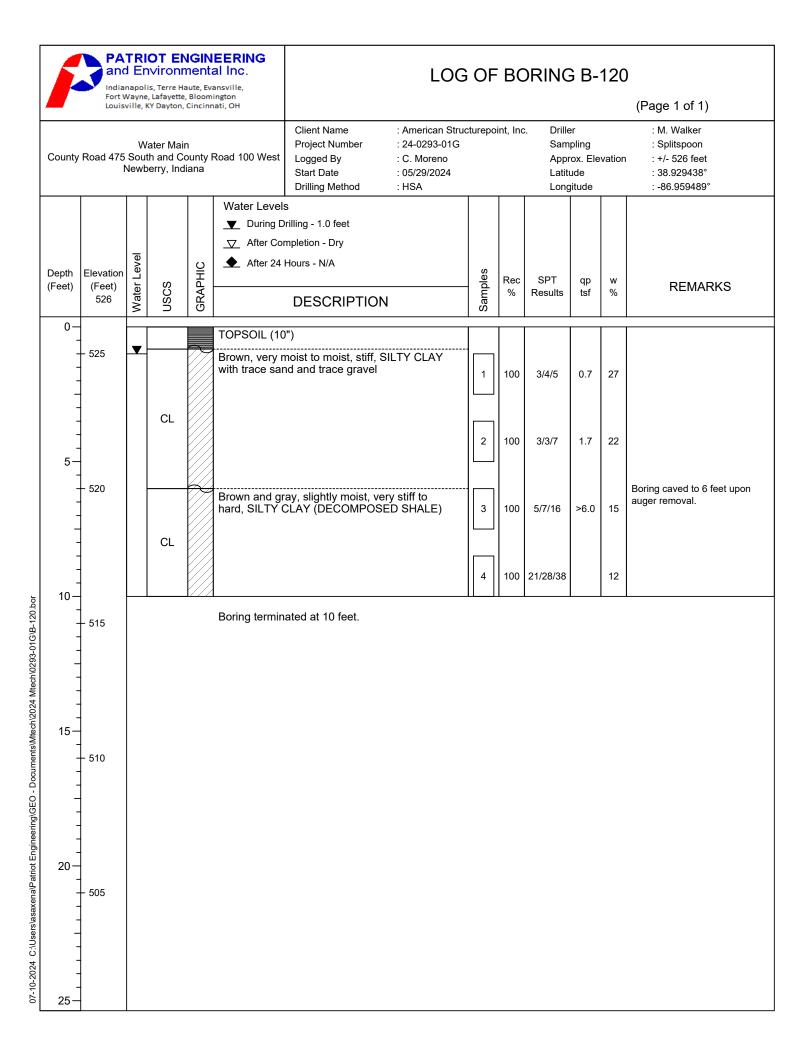


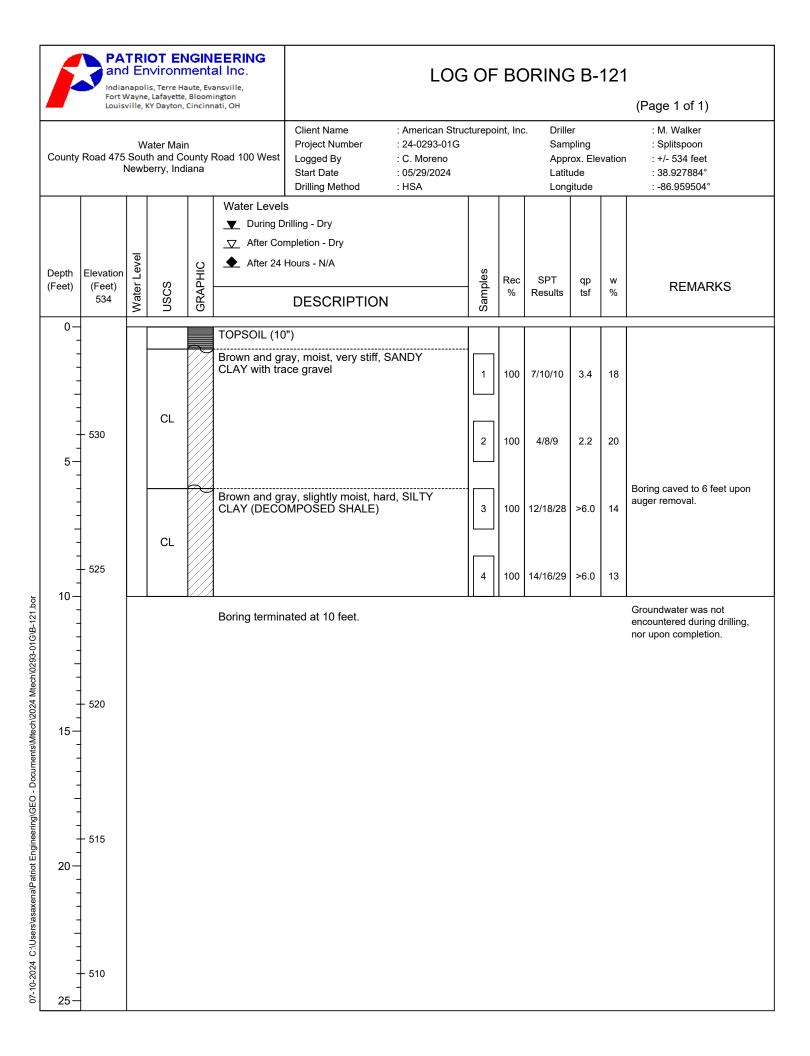


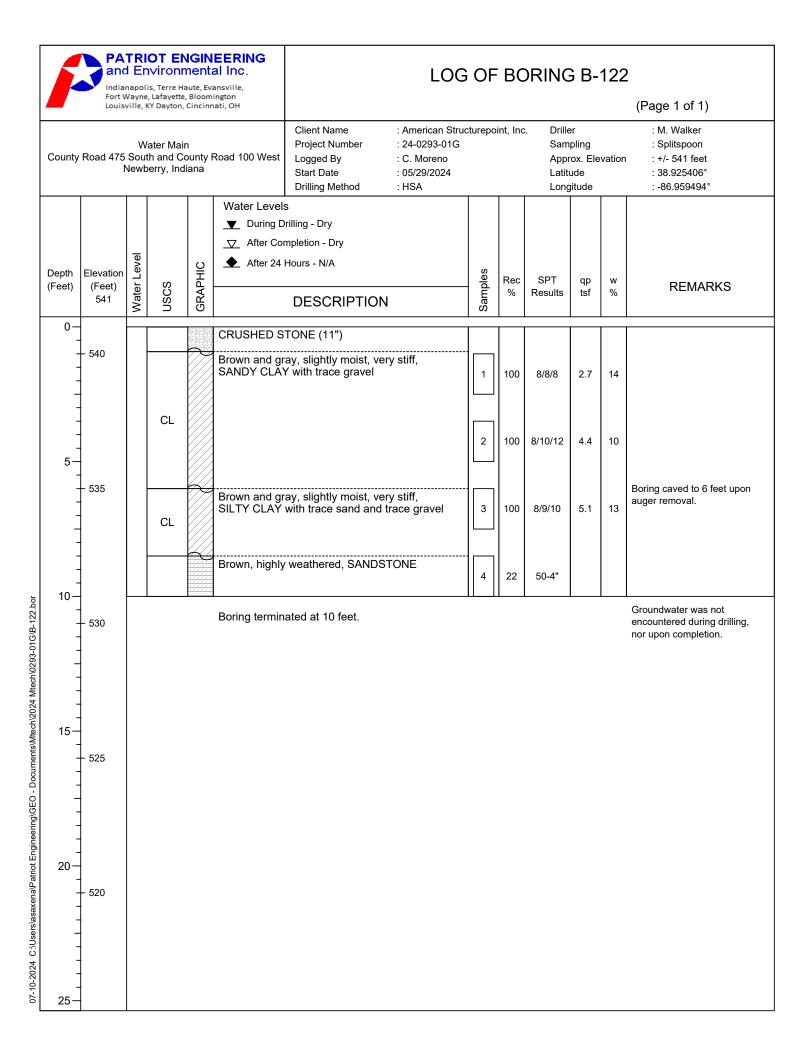


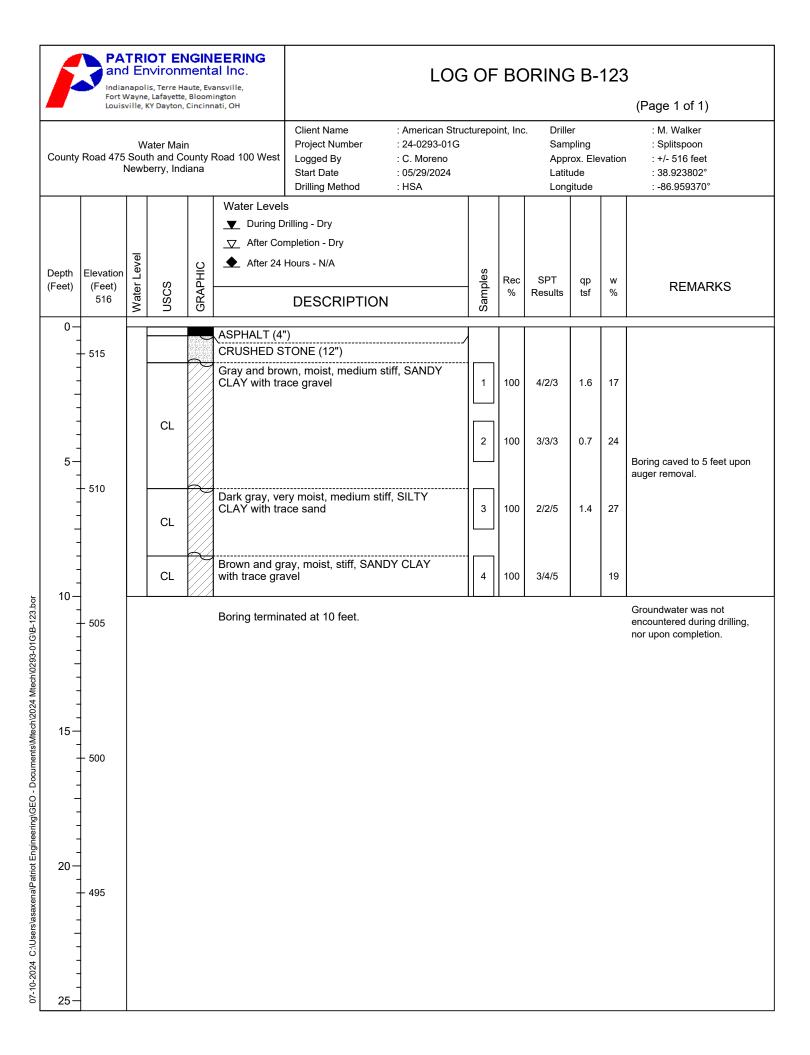


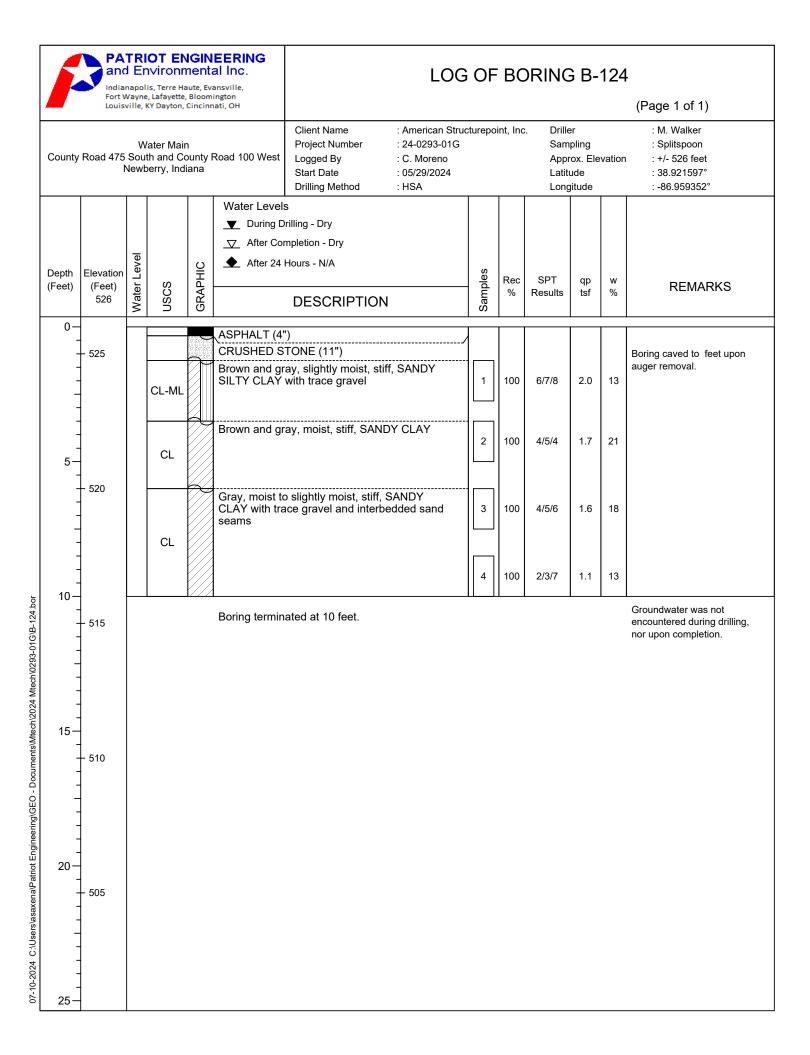


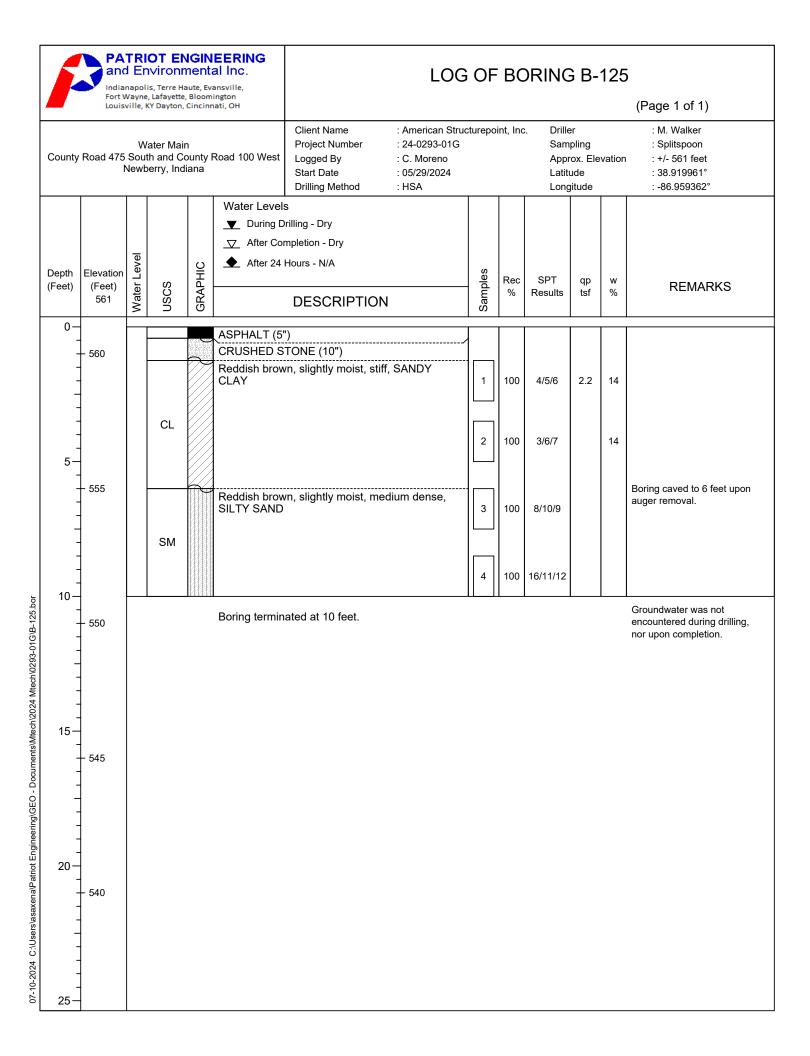


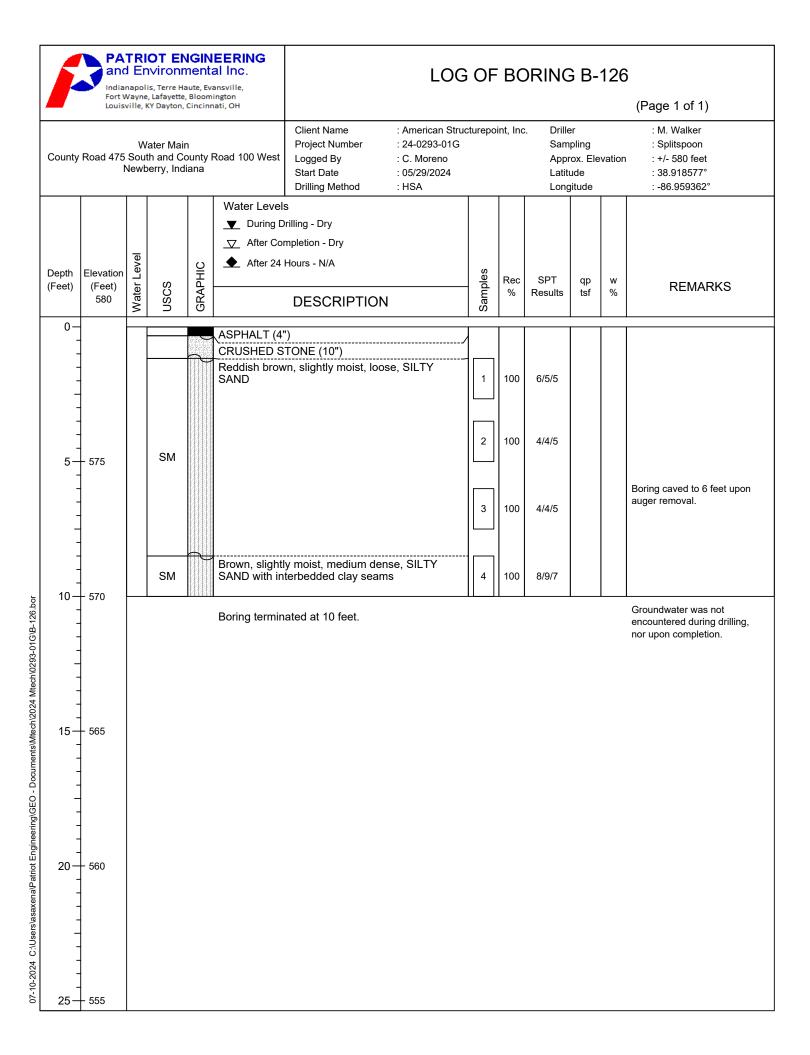


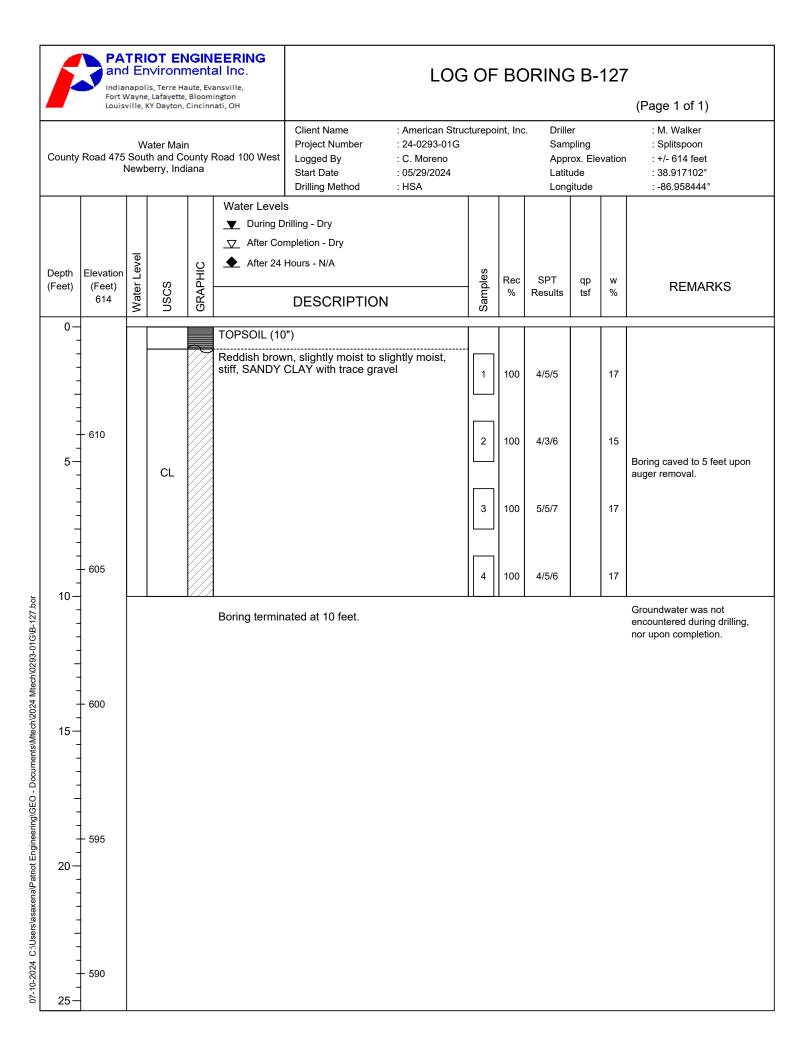














Density

## **BORING LOG KEY**

## UNIFIED SOIL CLASSIFICATION SYSTEM FIELD CLASSIFICATION SYSTEM FOR SOIL EXPLORATION

#### NON COHESIVE SOILS

(Silt, Sand, Gravel and Combinations)

**Grain Size Terminology** 

	onony								
Very Loose Loose	-4 blows/ft. or less	Soil Frac	tion Particle Size	US Standard Sieve Size					
Medium Dense Dense	-11 to 30 blows/ft. -31 to 50 blows/ft.	Boulders Cobbles	Larger than 12" 3" to12"	Larger than 12" 3" to 12"					
Very Dense	-51 blows/ft. or more	Gravel: Coa		<sup>3</sup> ⁄ <sub>4</sub> " to 3" #4 to <sup>3</sup> ⁄ <sub>4</sub> "					
		Sand: Coa	rse 2.00mm to 4.76mm dium 0.42mm to 2.00mm	#10 to #4 #40 to #10 #200 to #40					
		Silt Clay	0.005mm to 0.074 mm Smaller than 0.005mm	Smaller than #200					

#### RELATIVE PROPORTIONS FOR SOILS

<b>Descriptive Term</b>	Percent
Trace	1 - 10
Little	11 - 20
Some	21 - 35
And	36 - 50

## **COHESIVE SOILS**

(Clay, Silt and Combinations)

Consistency	Unconfined Compressive Strength (tons/sq. ft.)	Field Identification (Approx.) SPT Blows/ft.				
Very Soft	Less than 0.25	0 - 2				
Soft	0.25 – < 0.5	3 - 4				
Medium Stiff	0.5 - < 1.0	5 - 8				
Stiff	1.0 - < 2.0	9 -15				
Very Stiff	2.0 - < 4.0	16 - 30				
Hard	Over 4.0	> 30				

Classification on logs are made by visual inspection.

<u>Standard Penetration Test</u> - Driving a 2.0" O.D.,  $1^{3/8}$ " I.D., sampler a distance of 1.0 foot into undisturbed soil with a 140 pound hammer free falling a distance of 30.0 inches. It is customary for **Patriot** to drive the spoon 6.0 inches to seat into undisturbed soil, then perform the test. The number of hammer blows for seating the spoon and making the tests are recorded for each 6.0 inches of penetration on the drill log (Example - 6/8/9). The standard penetration test results can be obtained by adding the last two figures (i.e. 8 + 9 = 17 blows/ft.).

<u>Strata Changes</u> - In the column "Soil Descriptions" on the drill log the horizontal lines represent strata changes. A solid line (——————————) represents an actually observed change, a dashed line (- - - - - -) represents an estimated change.

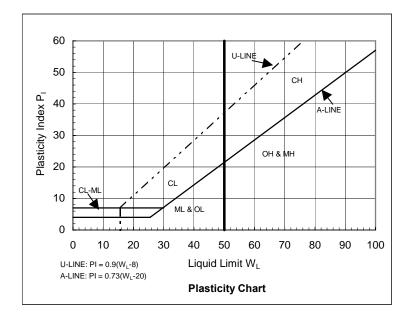
<u>Groundwater</u> observations were made at the times indicated. Porosity of soil strata, weather conditions, site topography, etc., may cause changes in the water levels indicated on the logs.

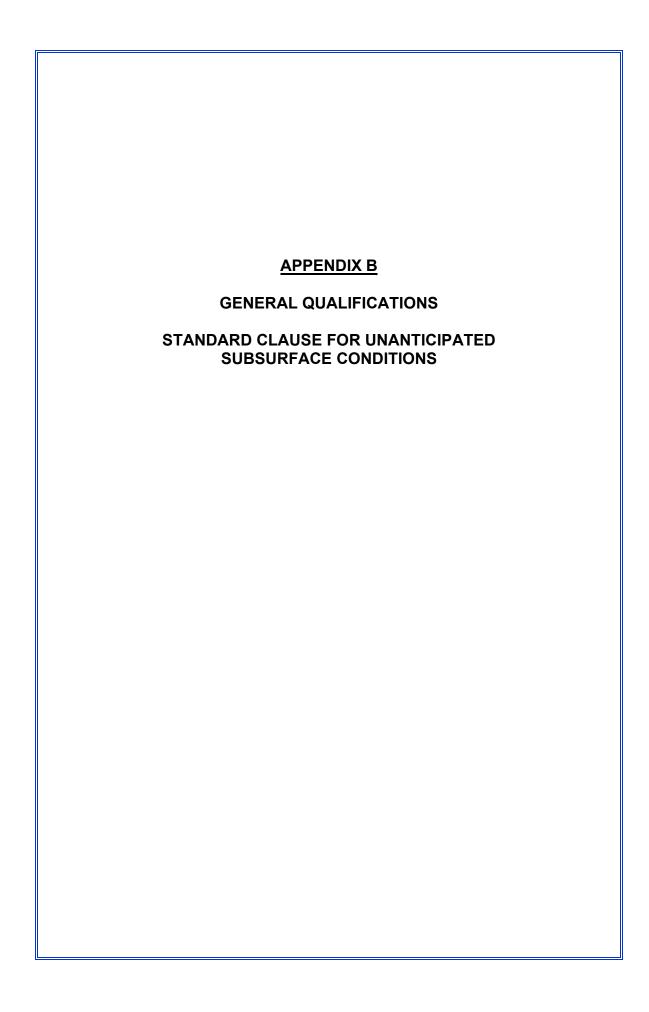
*Groundwater symbols*: ▼-observed groundwater elevation, encountered during drilling; ∇-observed groundwater elevation upon completion of boring.



# **Unified Soil Classification System**

Major Divisions			Group Symbol		Typical Names	Classification Criteria for Coarse-Gra		-Grained Soils	
	varse No. 4	Clean gravels (little or no fines)	GW		Well-graded gravels, gravel-sand mixtures, little or no fines	$C_{U} \ge 4$ $1 \le C_{C} \le 3$	Cu = -	) <sub>60</sub>	$C_{C} = \frac{D_{30}^{2}}{D_{10}D_{60}}$
s r than No. 200)	Gravels an half of cc larger than eve size)	Clean (little fir		GP	Poorly graded gravels, gravel-sand mixtures, little or no fines		Not meeting all gradation requirements for GW ( $C_U$ < 4 or 1 > $C_C$ > 3)		
	Gravels (more than half of coarse fraction is larger than No. 4 sieve size)	Gravels with fines (appreciable amount of fines)	GM	<u>d</u> u	Silty gravels, gravel-sand-silt mixtures	Atterberg limits A line or P <sub>I</sub> -			ove A line with  4 < P <sub>1</sub> < 7
ined soil	(mo fracti	Gravels v fines (apprecia amount fines)		GC	Clayey gravels, gravel-sand-clay mixtures		Atterberg limits above requiring u		oorderline cases iring use of dual symbols
Coarse-grained soils f of material is larger	arse No. 4	of coarse rthan No. 4 ze) Clean sands (little or no fines)		SW	Well-graded sands, gravelly sands, little or no fines	$C_{U} \ge 6$ $1 \le C_{C} \le 3$	$C_{U} \ge 6$ $1 \le C_{C} \le 3$ $C_{U} = \frac{C}{C}$		$C_{C} = \frac{(D_{30})^2}{D_{10} D_{60}}$
Coarse-grained soils (more than half of material is larger than No. 200)	Sands (more than half of coarse fraction is smaller than No. 4 sieve size)	Clean (little fin		SP	Poorly graded sands, gravelly sands, little or no fines		ng all grada W (C∪ < 6		irements for 3)
		Sands with fines (appreciable amount of fines)	SM	<u>d</u> u	Silty sands, sand-silt mixtures	Atterberg limits below A line or P <sub>I</sub> < 4		Limits plotting in hatched zone with 4 ≤ P₁ ≤ 7 are borderline cases	
	(mc fracti	Sands with fines (appreciable amount of fines)	SC SC		Clayey sands, sand-clay mixtures	Attorborg limita abovo			iring use of dual
Fine-grained soils (more than half of material is smaller than No. 200)	sk	ML		Inorganic silts and very fine sands, rock flour, silty or clayey fine sands, or clayey silts with slight plasticity	grain size cu	grain size curve.			
	Silt and clays (liquid limit <50)		CL		Inorganic clays of low to medium plasticity, gravelly clays, sandy clays, silty clays, lean clays	than 200 sieve size), coarse-grained soils a classified as follows: Less than 5% - GW, GP, SW, SP More than 12% - GM, GC, SM, SC			
d soils s smaller	0,	OL		Organic silts and organic silty clays of low		ng dual symbols			
Fine-grained soils of material is small	Silts and clays (liquid limit >50)		МН		Inorganic silts, micaceous or diatomaceous fine sandy or silty soils, elastic silts				
Fin alf of m			СН		Inorganic clays or high plasticity, fat clays				
than h	Silis	(liqu	ОН		Organic clays of medium to high plasticity, organic silts				
(more Highly organic soils			Highly organic soils		Peat and other highly organic soils				





## **GENERAL QUALIFICATIONS**

## of Patriot Engineering's Geotechnical Engineering Investigation

This report has been prepared at the request of our client for his use on this project. Our professional services have been performed, findings obtained, and recommendations prepared in accordance with generally accepted geotechnical engineering principles and practices. This warranty is in lieu of all other warranties either expressed or implied.

The scope of our services did not include any environmental assessment or investigation for the presence or absence of wetlands, hazardous or toxic materials in the soil, groundwater, or surface water within or beyond the site studied. Any statements in this report or on the test borings logs regarding vegetation types, odors or staining of soils, or other unusual conditions observed are strictly for the information of our client and the owner.

This report may not contain sufficient information for purposes of other parties or other uses. This company is not responsible for the independent conclusions, opinions or recommendations made by others based on the field and laboratory data presented in this report. Should there be any significant differences in structural arrangement, loading or location of the structure, our analysis should be reviewed.

The recommendations provided herein were developed from the information obtained in the test borings, which depict subsurface conditions only at specific locations. The analysis, conclusions, and recommendations contained in our report are based on site conditions as they existed at the time of our exploration. Subsurface conditions at other locations may differ from those occurring at the specific drill sites. The nature and extent of variations between borings may not become evident until the time of construction. If, after performing on-site observations during construction and noting the characteristics of any variation, substantially different subsurface conditions from those encountered during our explorations are observed or appear to be present beneath excavations, we must be advised promptly so that we can review these conditions and reconsider our recommendations where necessary.

If there is a substantial lapse of time between the submission of our report and the start of work at the site, or if conditions have changed due to natural causes or construction operations at or adjacent to the site, we urge that our report be reviewed to determine the applicability of the conclusions and recommendations considering the changed conditions and time lapse.

We urge that Patriot be retained to review those portions of the plans and specifications that pertain to earthwork and foundations to determine whether they are consistent with our recommendations. In addition, we are available to observe construction, particularly the compaction of structural backfill and preparation of the foundations, and such other field observations as may be necessary.

In order to fairly consider changed or unexpected conditions that might arise during construction, we recommend the following verbiage (Standard Clause for Unanticipated Subsurface Conditions) be included in the project contract.

## STANDARD CLAUSE FOR UNANTICIPATED SUBSURFACE CONDITIONS

"The owner has had a subsurface exploration performed by a soils consultant, the results of which are contained in the consultant's report. The consultant's report presents his conclusions on the subsurface conditions based on his interpretation of the data obtained in the exploration. The contractor acknowledges that he has reviewed the consultant's report and any addenda thereto, and that his bid for earthwork operations is based on the subsurface conditions as described in that report. It is recognized that a subsurface exploration may not disclose all conditions as they actually exist and further, conditions may change, particularly groundwater conditions, between the time of a subsurface exploration and the time of earthwork operations. In recognition of these facts, this clause is entered in the contract to provide a means of equitable additional compensation for the contractor if adverse unanticipated conditions are encountered and to provide a means of rebate to the owner if the conditions are more favorable than anticipated.

At any time during construction operations that the contractor encounters conditions that are different than those anticipated by the soils consultant's report, he shall immediately (within 24 hours) bring this fact to the owner's attention. If the owner's representative on the construction site observes subsurface conditions which are different than those anticipated by the consultant's report, he shall immediately (within 24 hours) bring this fact to the contractor's attention. Once a fact of unanticipated conditions has been brought to the attention of either the owner or the contractor, and the consultant has concurred, immediate negotiations will be undertaken between the owner and the contractor to arrive at a change in contract price for additional work or reduction in work because of the unanticipated conditions. The contract agrees that the following unit prices would apply for additional or reduced work under the contract. For changed conditions for which unit prices are not provided, the additional work shall be paid for on a time and materials basis."

Another example of a changed conditions clause can be found in paper No. 4035 by Robert F. Borg, published in <u>ASCE Construction Division Journal</u>, No. CO2, September 1964, page 37.